

Lecture 22: Finite element method: Axial vibrations of bars

Reading materials: Section 9.1

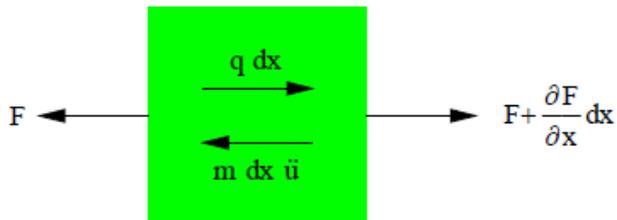
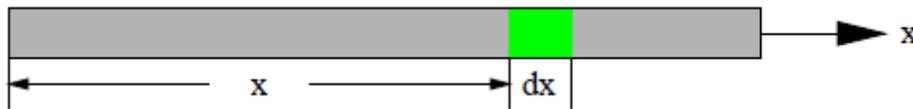
1. Introduction

● Discretization

● Assembly and solution

2. Governing equations

● Axial vibrations of a long slender bar



$$\frac{\partial}{\partial x} \left(A E \frac{\partial u}{\partial x} \right) + q = m \ddot{u}$$

● initial conditions

$$u(x, 0) = u_0 \quad \dot{u}(x, 0) = v_0$$

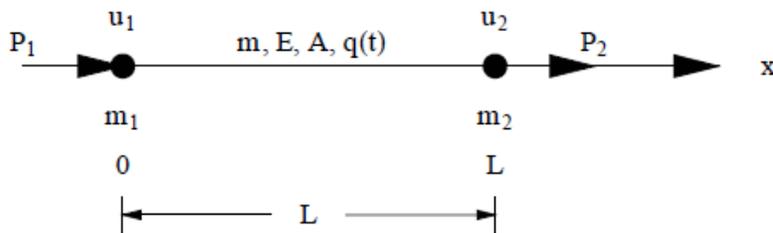
🟢 boundary conditions

$$u(x_\ell, t) = u_{x_\ell} \quad \text{or} \quad A(x_\ell) E(x_\ell) \frac{\partial u(x_\ell, t)}{\partial x} = F_{x_\ell}(t); \quad x_\ell \rightarrow \text{right end of the bar}$$

$$-A(x_0) E(x_0) \frac{\partial u(x_0, t)}{\partial x} = F_{x_0}(t); \quad x_0 \rightarrow \text{left end of the bar}$$

$$u(x_0, t) = u_{x_0}$$

3. Axial vibration element



🟢 Finite element approximation

$$y(x, t) = \sum_{i=1}^2 N_i(x) u_i(t) \equiv \mathbf{N}^T \mathbf{u}$$

$$y(\mathbf{x}) = a_0 + x a_1$$

$$y(0) = u_1 \implies a_0 = u_1$$

$$y(L) = u_2 \implies a_0 + L a_1 = u_2$$

$$\left\{ a_1 \rightarrow -\frac{u_1 - u_2}{L}, a_0 \rightarrow u_1 \right\}$$

$$y(\mathbf{x}) = u_1 - \frac{x(u_1 - u_2)}{L} = \left(1 - \frac{x}{L}\right) u_1 + \frac{x u_2}{L}$$

$$\mathbf{N}^T = \left\{ 1 - \frac{x}{L}, \frac{x}{L} \right\}$$

$$\frac{dy(x,t)}{dx} = \sum_{i=1}^2 \frac{dN_i}{dx} u_i(t) \equiv \mathbf{B}^T \mathbf{u}$$

$$\mathbf{B}^T = \left\{ -\frac{1}{L}, \frac{1}{L} \right\}$$

4. Energy

• kinetic energy due to distributed mass

$$T_d = \frac{1}{2} \int_0^L m \left(\frac{dy(x,t)}{dt} \right)^2 dx = \frac{1}{2} \dot{\mathbf{u}}^T \int_0^L m \mathbf{N} \mathbf{N}^T dx \dot{\mathbf{u}} \equiv \frac{1}{2} \dot{\mathbf{u}}^T \mathbf{m}_d \dot{\mathbf{u}}$$

$$\mathbf{m}_d = \int_0^L m \mathbf{N} \mathbf{N}^T dx = \int_0^L \begin{pmatrix} m \left(1 - \frac{x}{L}\right)^2 & \frac{mx \left(1 - \frac{x}{L}\right)}{L} \\ \frac{mx \left(1 - \frac{x}{L}\right)}{L} & \frac{mx^2}{L^2} \end{pmatrix} dx = \begin{pmatrix} \frac{Lm}{3} & \frac{Lm}{6} \\ \frac{Lm}{6} & \frac{Lm}{3} \end{pmatrix}$$

• kinetic energy due to concentrated mass

$$T_c = \frac{1}{2} (m_1 u_1^2 + m_2 u_2^2) = \frac{1}{2} \dot{\mathbf{u}}^T \mathbf{m}_c \dot{\mathbf{u}} \quad \mathbf{m}_c = \begin{pmatrix} m_1 & 0 \\ 0 & m_2 \end{pmatrix}$$

• Total kinetic energy

$$T = T_d + T_c = \frac{1}{2} \dot{\mathbf{u}}^T (\mathbf{m}_d + \mathbf{m}_c) \dot{\mathbf{u}} \equiv \frac{1}{2} \dot{\mathbf{u}}^T \mathbf{m} \dot{\mathbf{u}}$$

$$\mathbf{m} = \mathbf{m}_d + \mathbf{m}_c = \begin{pmatrix} \frac{Lm}{3} + m_1 & \frac{Lm}{6} \\ \frac{Lm}{6} & \frac{Lm}{3} + m_2 \end{pmatrix}$$

● Strain energy

$$U_s = \frac{1}{2} \int_0^L EA \left(\frac{dy(x,t)}{dx} \right)^2 dx = \frac{1}{2} \mathbf{u}^T \int_0^L EA \mathbf{B} \mathbf{B}^T dx \mathbf{u} \equiv \frac{1}{2} \mathbf{u}^T \mathbf{k} \mathbf{u}$$

$$\mathbf{k} = \int_0^L EA \mathbf{B} \mathbf{B}^T dx = \int_0^L \begin{pmatrix} \frac{AE}{L^2} & -\frac{AE}{L^2} \\ -\frac{AE}{L^2} & \frac{AE}{L^2} \end{pmatrix} dx = \frac{AE}{L} \begin{pmatrix} 1 & -1 \\ -1 & 1 \end{pmatrix}$$

● work done by the distributed applied force

$$W_q = \int_0^L q(t) y(x, t) dx = \int_0^L q N^T dx \mathbf{u} \equiv \mathbf{r}_q^T \mathbf{u}$$

$$\mathbf{r}_q^T = \int_0^L q N^T dx = \int_0^L \left\{ q \left(1 - \frac{x}{L} \right), \frac{qx}{L} \right\} dx = \left\{ \frac{Lq}{2}, \frac{Lq}{2} \right\}$$

● work done by the concentrated loads

$$W_p = (p_1 u_1 + p_2 u_2) = \mathbf{r}_p^T \mathbf{u}$$

$$\mathbf{r}_p^T = (p_1 \quad p_2)^T$$

● Total work done

$$W = W_q + W_p = (\mathbf{r}_q + \mathbf{r}_p)^T \mathbf{u} \equiv \mathbf{r}^T \mathbf{u}$$

$$\mathbf{r} = \mathbf{r}_q + \mathbf{r}_p = \begin{pmatrix} \frac{Lq}{2} + p_1 \\ \frac{Lq}{2} + p_2 \end{pmatrix}$$

5. Equations of motion

$$U = U_s - W$$

$$\frac{d}{dt}(T + U) = 0 \implies \frac{d}{dt} \left[\frac{1}{2} \dot{\mathbf{u}}^T \mathbf{m} \dot{\mathbf{u}} + \frac{1}{2} \mathbf{u}^T \mathbf{k} \mathbf{u} - \mathbf{r}^T \mathbf{u} \right] = \dot{\mathbf{u}}^T [\mathbf{m} \ddot{\mathbf{u}} + \mathbf{k} \mathbf{u} - \mathbf{r}] = \mathbf{0}$$

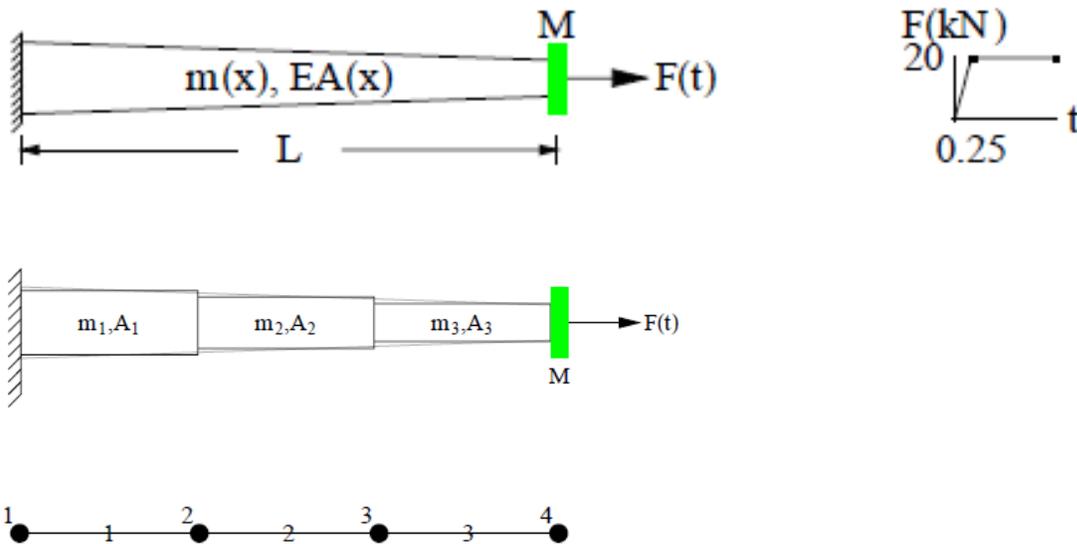
$$\mathbf{m} \ddot{\mathbf{u}}(t) + \mathbf{k} \mathbf{u}(t) = \mathbf{r}(t)$$

$$\begin{pmatrix} \frac{Lm}{3} + m_1 & \frac{Lm}{6} \\ \frac{Lm}{6} & \frac{Lm}{3} + m_2 \end{pmatrix} \begin{pmatrix} \ddot{u}_1 \\ \ddot{u}_2 \end{pmatrix} + \begin{pmatrix} \frac{EA}{L} & -\frac{EA}{L} \\ -\frac{EA}{L} & \frac{EA}{L} \end{pmatrix} \begin{pmatrix} u_1 \\ u_2 \end{pmatrix} = \begin{pmatrix} \frac{Lq}{2} + p_1 \\ \frac{Lq}{2} + p_2 \end{pmatrix}$$

6. Assembly and Solution

- The element equations are derived in the above with local coordinates.
- Assemble the element equations into the global equations based on the global coordinates.
- Apply the boundary conditions.
- Numerically solve the equations of motion.

7. Example 1



• General element equations

$$\begin{pmatrix} \frac{Lm}{3} & \frac{Lm}{6} \\ \frac{Lm}{6} & \frac{Lm}{3} \end{pmatrix} \begin{pmatrix} \ddot{u}_1 \\ \ddot{u}_2 \end{pmatrix} + \begin{pmatrix} \frac{EA}{L} & -\frac{EA}{L} \\ -\frac{EA}{L} & \frac{EA}{L} \end{pmatrix} \begin{pmatrix} u_1 \\ u_2 \end{pmatrix} = \begin{pmatrix} \frac{Lq}{2} \\ \frac{Lq}{2} \end{pmatrix}$$

• Element 1

$$\begin{pmatrix} 1.869 & 0.9345 \\ 0.9345 & 1.869 \end{pmatrix} \begin{pmatrix} \ddot{u}_1 \\ \ddot{u}_2 \end{pmatrix} + \begin{pmatrix} 490000 & -490000 \\ -490000 & 490000 \end{pmatrix} \begin{pmatrix} u_1 \\ u_2 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \end{pmatrix}$$

• Element 2

$$\begin{pmatrix} 1.335 & 0.6675 \\ 0.6675 & 1.335 \end{pmatrix} \begin{pmatrix} \ddot{u}_2 \\ \ddot{u}_3 \end{pmatrix} + \begin{pmatrix} 350000 & -350000 \\ -350000 & 350000 \end{pmatrix} \begin{pmatrix} u_2 \\ u_3 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \end{pmatrix}$$

Element 3

$$\begin{pmatrix} 0.801 & 0.4005 \\ 0.4005 & 0.801 \end{pmatrix} \begin{pmatrix} \ddot{u}_3 \\ \ddot{u}_4 \end{pmatrix} + \begin{pmatrix} 210000 & -210000 \\ -210000 & 210000 \end{pmatrix} \begin{pmatrix} u_3 \\ u_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \end{pmatrix}$$

Considering concentrated mass and force on node 4, the element equations for Element 3 could be written as:

$$\begin{pmatrix} 0.801 & 0.4005 \\ 0.4005 & 100.801 \end{pmatrix} \begin{pmatrix} \ddot{u}_3 \\ \ddot{u}_4 \end{pmatrix} + \begin{pmatrix} 210000 & -210000 \\ -210000 & 210000 \end{pmatrix} \begin{pmatrix} u_3 \\ u_4 \end{pmatrix} = \begin{pmatrix} 0 \\ F(t) \end{pmatrix}$$

Form of global equations

$$\begin{pmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{pmatrix} \begin{pmatrix} \ddot{u}_1 \\ \ddot{u}_2 \\ \ddot{u}_3 \\ \ddot{u}_4 \end{pmatrix} + \begin{pmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{pmatrix} \begin{pmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ 0 \end{pmatrix}$$

Assemble of elements

Element 1

$$\mathbf{lm} = (1 \ 2)$$

$$\text{Assembly locations for } k \ \& \ m \Rightarrow \begin{pmatrix} [1, 1] & [1, 2] \\ [2, 1] & [2, 2] \end{pmatrix} \text{ and for } r \Rightarrow \begin{pmatrix} 1 \\ 2 \end{pmatrix}$$

$$k = \begin{pmatrix} 490000. & -490000. & 0 & 0 \\ -490000. & 490000. & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{pmatrix}$$

Element 2

$$lm = (2 \ 3)$$

$$\text{Assembly locations for } k \ \& \ m \Rightarrow \begin{pmatrix} [2, 2] & [2, 3] \\ [3, 2] & [3, 3] \end{pmatrix} \text{ and for } r \Rightarrow \begin{pmatrix} 2 \\ 3 \end{pmatrix}$$

$$k = \begin{pmatrix} 490000. & -490000. & 0 & 0 \\ -490000. & 840000. & -350000. & 0 \\ 0 & -350000. & 350000. & 0 \\ 0 & 0 & 0 & 0 \end{pmatrix}$$

Element 3

$$lm = (3 \ 4)$$

$$\text{Assembly locations for } k \ \& \ m \Rightarrow \begin{pmatrix} [3, 3] & [3, 4] \\ [4, 3] & [4, 4] \end{pmatrix} \text{ and for } r \Rightarrow \begin{pmatrix} 3 \\ 4 \end{pmatrix}$$

$$m = \begin{pmatrix} 1.869 & 0.9345 & 0 & 0 \\ 0.9345 & 3.204 & 0.6675 & 0 \\ 0 & 0.6675 & 2.136 & 0.4005 \\ 0 & 0 & 0.4005 & 100.801 \end{pmatrix}$$

$$k = \begin{pmatrix} 490000. & -490000. & 0 & 0 \\ -490000. & 840000. & -350000. & 0 \\ 0 & -350000. & 560000. & -210000. \\ 0 & 0 & -210000. & 210000. \end{pmatrix}$$

Global equations of motion

$$\begin{pmatrix} 1.869 & 0.9345 & 0 & 0 \\ 0.9345 & 3.204 & 0.6675 & 0 \\ 0 & 0.6675 & 2.136 & 0.4005 \\ 0 & 0 & 0.4005 & 100.801 \end{pmatrix} \begin{pmatrix} \ddot{u}_1 \\ \ddot{u}_2 \\ \ddot{u}_3 \\ \ddot{u}_4 \end{pmatrix} + \begin{pmatrix} 490\,000. & -490\,000. & 0 & 0 \\ -490\,000. & 840\,000. & -350\,000. & 0 \\ 0 & -350\,000. & 560\,000. & -210\,000. \\ 0 & 0 & -210\,000. & 210\,000. \end{pmatrix} \begin{pmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ F(t) \end{pmatrix}$$

Apply BCs

$$u_1 = \ddot{u}_1 = 0$$

$$\begin{pmatrix} 3.204 & 0.6675 & 0 \\ 0.6675 & 2.136 & 0.4005 \\ 0 & 0.4005 & 100.801 \end{pmatrix} \begin{pmatrix} \ddot{u}_2 \\ \ddot{u}_3 \\ \ddot{u}_4 \end{pmatrix} + \begin{pmatrix} 840\,000. & -350\,000. & 0 \\ -350\,000. & 560\,000. & -210\,000. \\ 0 & -210\,000. & 210\,000. \end{pmatrix} \begin{pmatrix} u_2 \\ u_3 \\ u_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ F(t) \end{pmatrix}$$

Note: for nonzero BCs

$$\begin{pmatrix} 3.204 & 0.6675 & 0 \\ 0.6675 & 2.136 & 0.4005 \\ 0 & 0.4005 & 100.801 \end{pmatrix} \begin{pmatrix} \ddot{u}_2 \\ \ddot{u}_3 \\ \ddot{u}_4 \end{pmatrix} + \begin{pmatrix} 840\,000. & -350\,000. & 0 \\ -350\,000. & 560\,000. & -210\,000. \\ 0 & -210\,000. & 210\,000. \end{pmatrix} \begin{pmatrix} u_2 \\ u_3 \\ u_4 \end{pmatrix} \\ = \begin{pmatrix} 0 \\ 0 \\ F(t) \end{pmatrix} - \begin{pmatrix} 0.9345 \\ 0 \\ 0 \end{pmatrix} \ddot{u}_1 - \begin{pmatrix} -490\,000. \\ 0 \\ 0 \end{pmatrix} u_1$$

Solutions

Newmark's method