

## Lecture 13: Frequency Domain Solution

Reading materials: Sections 4.6 and 4.7

### 1. Steady-state solution for complex forcing function

• Equation of motion (Single degree of freedom)

$$m \ddot{u}(t) + c \dot{u}(t) + k u(t) = F e^{i\Omega t}; \quad u(0) = u_0 \quad \dot{u}(0) = \dot{u}_0$$

• For steady-state solution, assume the solution

$$u(t) = U e^{i\Omega t}$$

• Get the solution

$$-\Omega^2 m U e^{i\Omega t} + i \Omega c U e^{i\Omega t} + k U e^{i\Omega t} = F e^{i\Omega t}$$

$$U = [-\Omega^2 m + i \Omega c + k]^{-1} F$$

$$\dot{u}(t) = i \Omega U e^{i\Omega t} = \dot{U} e^{i\Omega t} \implies \dot{U} \equiv i \Omega U$$

$$\ddot{u}(t) = -\Omega^2 U e^{i\Omega t} = \ddot{U} e^{i\Omega t} \implies \ddot{U} \equiv -\Omega^2 U$$

Not time derivatives.

• For Periodic loading, using Fourier transform

$$f(t) = \sum_{n=-p}^p F_n e^{in\Omega t}$$

$$F_n = \frac{\Omega}{2\pi} \int_0^T f(t) e^{-in\Omega t} dt; \quad n = 0, 1, 2, \dots; \quad F_{-n} = \text{Conj}(F_n)$$

For each component, the solution amplitude is

$$U_n = [-n^2 \Omega^2 m + i n \Omega c + k]^{-1} F_n \equiv H_n F_n$$

$$H_n = \frac{1}{-n^2 \Omega^2 m + i n \Omega c + k}$$

here

$$\text{Natural frequency, } \omega = \sqrt{k/m} \implies \frac{m}{k} = \frac{1}{\omega^2}$$

$$\text{Damping ratio, } \xi = \frac{c}{2m\omega} \implies \frac{c}{k} = \frac{2m\omega\xi}{k} = \frac{2\xi}{\omega}$$

$$\text{Frequency ratio, } r_n = \frac{n\Omega}{\omega}$$

Therefore,

$$H_n = \frac{1}{k(-r_n^2 + 2i r_n \xi + 1)}$$

Complete solution

$$u(t) = \sum_{n=-p}^p U_n e^{in\Omega t}$$

Summary of the procedure

- a. Use the direct Fourier transform to determine the complex frequency amplitudes for the applied force – transform force into the frequency domain

$$F_n = \frac{\Omega}{2\pi} \int_0^T f(t) e^{-in\Omega t} dt; \quad n = 0, 1, 2, \dots; \quad F_{-n} = \text{Conj}(F_n)$$

b. Determine the complex frequency amplitudes of the solution

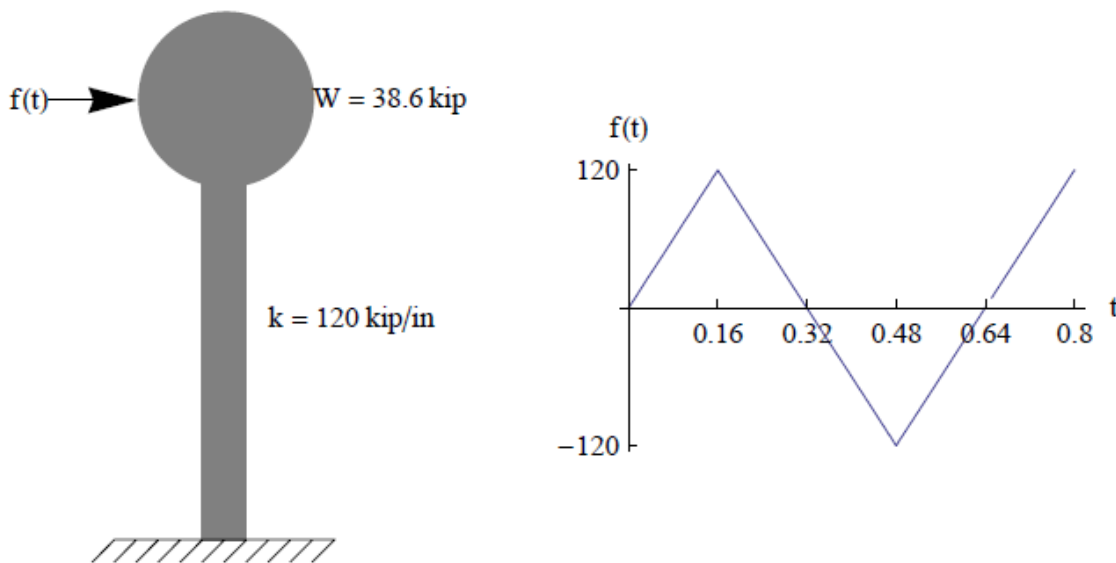
$$H_n = \frac{1}{-n^2 \Omega^2 m + i n \Omega c + k} = \frac{1}{k(-r_n^2 + 2i r_n \xi + 1)}$$

$$U_n = H_n F_n; \quad \dot{U}_n = i n \Omega H_n F_n; \quad \ddot{U}_n = -n^2 \Omega^2 H_n F_n$$

c. Use the inverse Fourier transform to take the frequency amplitudes of the solution back into the time domain

$$u(t) = \sum_{n=-p}^p U_n e^{i n \Omega t}; \quad \dot{u}(t) = \sum_{n=-p}^p i n \Omega U_n e^{i n \Omega t}; \quad \ddot{u}(t) = -\sum_{n=-p}^p n^2 \Omega^2 U_n e^{i n \Omega t}$$

🟢 Example: Determine the steady-state motion of the water tower when it is subjected to a triangular periodic load. 10% damping



$$\text{Load period, } T = 0.64 \implies \Omega = \frac{2\pi}{0.64} = 9.81748$$

$$f(t) = \begin{cases} 750 \cdot (t - 0.16) + 120 & t \leq 0.16 \\ -750 \cdot (t - 0.48) - 120 & t \leq 0.48; \\ 750 \cdot (t - 0.64) & t \leq 0.64 \end{cases} \quad \Omega = 9.81748$$

$$m = 0.1; \quad k = 120.; \quad \omega = 34.641$$

$$\xi = 0.1; \quad c = 0.69282;$$

$$F_{-3} = (1/0.64) \int_0^{0.64} f(t) e^{(-i)(-3)(9.81748)t} dt = -5.4038 i; \quad H_{-3} = 0.0218456 + 0.0134042 i; \quad U_{-3} = 0.0724338 - 0.118049 i$$

$$F_{-2} = (1/0.64) \int_0^{0.64} f(t) e^{(-i)(-2)(9.81748)t} dt = 0; \quad H_{-2} = 0.0119447 + 0.00199504 i; \quad U_{-2} = 0$$

$$F_{-1} = (1/0.64) \int_0^{0.64} f(t) e^{(-i)(-1)(9.81748)t} dt = 48.6342 i; \quad H_{-1} = 0.00902683 + 0.000556336 i; \quad U_{-1} = -0.0270569 + 0.439012 i$$

$$F_0 = (1/0.64) \int_0^{0.64} f(t) e^{(-i)(0)(9.81748)t} dt = 0; \quad H_0 = 0.00833333; \quad U_0 = 0$$

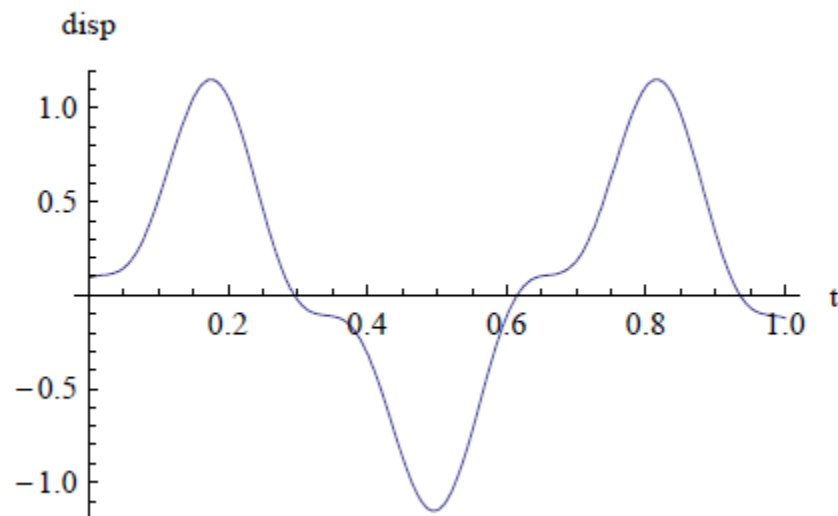
$$F_1 = (1/0.64) \int_0^{0.64} f(t) e^{(-i)(1)(9.81748)t} dt = -48.6342 i; \quad H_1 = 0.00902683 - 0.000556336 i; \quad U_1 = -0.0270569 - 0.439012 i$$

$$F_2 = (1/0.64) \int_0^{0.64} f(t) e^{(-i)(2)(9.81748)t} dt = 0; \quad H_2 = 0.0119447 - 0.00199504 i; \quad U_2 = 0$$

$$F_3 = (1/0.64) \int_0^{0.64} f(t) e^{(-i)(3)(9.81748)t} dt = 5.4038 i; \quad H_3 = 0.0218456 - 0.0134042 i; \quad U_3 = 0.0724338 + 0.118049 i$$

$$u(t) = (-0.0270569 + 0.439012 i) e^{-9.81748 it} - (0.0270569 + 0.439012 i) e^{9.81748 it}$$

$$+ (0.0724338 - 0.118049 i) e^{-29.4524 it} + (0.0724338 + 0.118049 i) e^{29.4524 it}$$



## 2. Using discrete Fourier Transform for numerical force data

Use the direct Discrete Fourier Transform to compute the complex frequency amplitudes for the applied force

$$F_n = \frac{1}{N} \sum_{s=0}^{N-1} f_s e^{-i(2\pi s n/N)}; \quad n = 0, 1, 2, \dots, N-1$$

Determine the complex frequency amplitudes of the solution

$$H_n = \frac{1}{-n^2 \Omega^2 m + i n \Omega c + k}$$

$$U_n = H_n F_n; \quad \dot{U}_n = i n \Omega H_n F_n; \quad \ddot{U}_n = -n^2 \Omega^2 H_n F_n$$

Use the inverse Fourier transform to transform frequency amplitudes of the solution back into the time domain

$$u_s = \sum_{n=0}^{N-1} U_n e^{i(2\pi s n/N)}; \quad s = 0, 1, 2, \dots, N$$

$$\dot{u}_s = \sum_{n=0}^{N-1} \dot{U}_n e^{i(2\pi s n/N)}; \quad s = 0, 1, 2, \dots, N$$

$$\ddot{u}_s = \sum_{n=0}^{N-1} \ddot{U}_n e^{i(2\pi s n/N)}; \quad s = 0, 1, 2, \dots, N$$

## 3. General Non-Periodic loading

General loading can be treated in the frequency domain approach by extending the load period to include a large interval of zero force to the end of the actual force. Mathematically the load is still periodic.

The method is used only for large duration loading where the initial built-up of the loading is relatively slow.

#### 4. Impulsive force

- A nonperiodic exciting force usually has a magnitude that varies with time; it acts for a specified period of time and then stops.
- The simplest form is the impulsive force – a force that has a large magnitude  $F$  and acts for a very short period of time.

## 5. Multiple degrees of freedom frequency domain solution

### • Coupled Equation of motion

$$m \ddot{u} + c \dot{u} + k u = f(t)$$

Rayleigh damping

$$c = \alpha m + \beta k$$

### • Undamped free vibration modes

$$k \phi_i = \lambda_i m \phi_i; \quad i = 1, 2, \dots, n$$

Mass orthogonality:

$$\phi_j^T m \phi_i = 0; \quad i \neq j$$

Stiffness orthogonality:

$$\phi_j^T k \phi_i = 0; \quad i \neq j$$

### • Uncoupled equations

Modal coordinates

$$\mathbf{z} = (z_1 \quad z_2 \quad \dots \quad z_n)^T$$

$$\mathbf{u}(t) = \sum_i z_i(t) \phi_i$$

Damped modal equations

$$M_i \ddot{z}_i(t) + (\alpha M_i + \beta K_i) \dot{z}_i(t) + K_i z_i(t) = R_i; \quad i = 1, 2, \dots$$

$$z_i(0) = \frac{1}{M_i} (\phi_i^T m u^0); \quad \dot{z}_i(0) = \frac{1}{M_i} (\phi_i^T m v^0);$$

$$M_i = \phi_i^T m \phi_i; \quad K_i = \phi_i^T k \phi_i; \quad \omega_i = \sqrt{K_i/M_i}; \quad R_i = \phi_i^T F$$

$$C_i = \alpha M_i + \beta K_i$$

$$M_i \ddot{z}_i(t) + C_i \dot{z}_i(t) + K_i z_i(t) = R_i; \quad i = 1, 2, \dots$$

### ● Solution of modal equations

- a. Use the direct Discrete Fourier transform to compute the complex frequency amplitudes for the applied force

$$F_{i,n} = \frac{1}{N} \sum_{s=0}^{N-1} R_{i,s} e^{-i(2\pi s n/N)}; \quad n = 0, 1, 2, \dots, N-1$$

- b. Determine the complex frequency amplitudes of the solution

$$H_{i,n} = \frac{1}{-n^2 \Omega^2 M_i + i n \Omega C_i + K_i}$$

$$Z_{i,n} = H_{i,n} F_{i,n}; \quad \dot{Z}_{i,n} = i n \Omega H_{i,n} F_{i,n}; \quad \ddot{Z}_{i,n} = -n^2 \Omega^2 H_{i,n} F_{i,n}$$

- c. Use the Inverse Fourier transform to transform frequency amplitudes of the solution back into the time domain

$$z_{i,s} = \sum_{n=0}^{N-1} Z_{i,n} e^{i(2\pi s n/N)}; \quad s = 0, 1, 2, \dots, N$$

$$\dot{z}_{i,s} = \sum_{n=0}^{N-1} \dot{Z}_{i,n} e^{i(2\pi s n/N)}; \quad s = 0, 1, 2, \dots, N$$

$$\ddot{z}_{i,s} = \sum_{n=0}^{N-1} \ddot{Z}_{i,n} e^{i(2\pi s n/N)}; \quad s = 0, 1, 2, \dots, N$$

Final solution

$$u = \sum_i z_i \phi_i$$