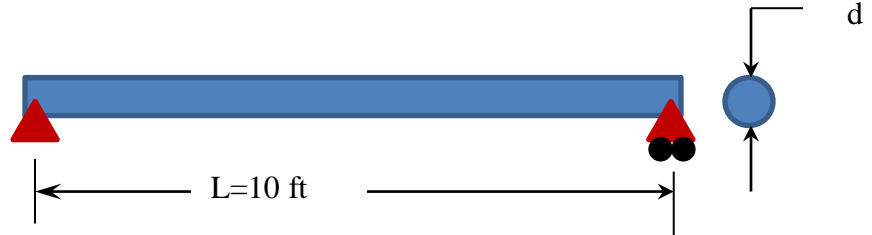


The University of Iowa  
57:019:BBB Intro. To Mechanics of Deformable Bodies  
Spring Semester 2011  
Quiz #3 Makeup Solution

**Problem 1 (33 pts):**

A solid steel rod of diameter  $d$  is simply supported as shown. The unit weight for steel is  $\gamma = 490 \text{ lbs} \cdot \text{ft}^{-3}$ . Determine the required diameter  $d$  of the rod so that the bending stress in the beam does not exceed 4 ksi. **Hint:** treat the weight of the beam as a uniformly distributed load  $w = \gamma A$  where  $A$  is the cross-sectional area of the beam. Recall that for a circular cross-section,  $I = J / 2$ .



**Solution:**

Let the uniform load acting on the beam be  $w = \gamma A$ , where  $A$  is the cross-sectional area of the beam, and  $\gamma$  is the unit weight. Here,  $w = \frac{490 \text{ lbs}}{\text{ft}^3} \cdot \pi c^2$

For a simply supported and uniformly loaded beam the end reactions will have magnitude  $wL/2$ . The maximum bending moment occurs at mid-span and has magnitude:

$$M_{\max} = \frac{wL^2}{8} = 490\pi c^2 * \frac{(10')^2}{8} = 19,242 \frac{\text{lbs}}{\text{ft}} c^2$$

At the location of maximum bending moment, the maximum bending stress is given by the flexure

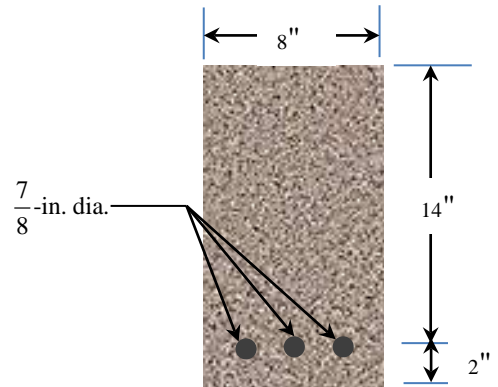
formula as follows: 
$$\sigma_{\max} = \frac{Mc}{I} = \left( 19,242 \frac{\text{lbs}}{\text{ft}} c^2 \right) * \frac{c}{\frac{\pi}{4} c^4} = 4 \text{ ksi} = 576 \text{ ksi}$$

Solving for  $c$  yields:  $c = 0.0425' = 0.514''$

There  $\boxed{d_{\text{reqd}} = 2c = 1.03''}$

**Problem 2 (33 pts):**

A concrete beam is reinforced by three steel rods placed as shown. For the concrete  $E=3,000\text{ksi}$ , and for the steel  $E=29,000\text{ksi}$ . If the allowable stress in the concrete is  $1.35\text{ksi}$  and that in the steel is  $20\text{ksi}$ , what is the largest positive bending moment that can be applied to the beam section?

**Solution:**

$$n = \frac{E_{st}}{E_c} = \frac{29}{3} = 9.67$$

$$A_{st} = 3 * \frac{\pi}{4} * \left(\frac{7''}{8}\right)^2 = 1.804\text{in}^2$$

$$nA_{st} = 17.44\text{in}^2$$

To locate the NA, moment of area above should be equal to transformed steel area below:

$$nA_{st}(d - h') = \frac{bh'^2}{2} \Rightarrow h' = 5.93''$$

$$I^* = \frac{b(h')^3}{12} + \frac{b(h')^3}{4} + nA_{st}(d - h')^2 = 1692\text{in}^4$$

$$(\sigma_{all})_{conc} = 1.35\text{ksi} = \frac{(M_{allow})_{conc} h'}{I^*} \Rightarrow (M_{allow})_{conc} = \frac{(\sigma_{all})_{conc} I^*}{h'} = 385\text{k} \cdot \text{in}$$

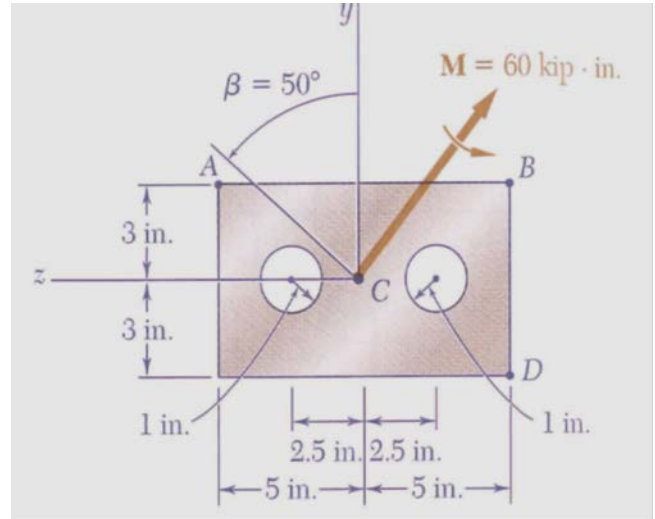
$$(\sigma_{all})_{st} = 20\text{ksi} = \frac{n(M_{allow})_{st}(d - h')}{I^*} \Rightarrow (M_{allow})_{st} = \frac{(\sigma_{all})_{st} I^*}{n(d - h')} = 434\text{k} \cdot \text{in}$$

$$M_{allowable} = \min\{385, 434\} = 385\text{k} \cdot \text{in}$$

**Problem 3 (34 pts):**

The moment of 60 kip-in acts on the cross-section shown and makes an angle of  $40^\circ$  with respect to the section's y-axis. Calculate:

- $I_{zz}$  and  $I_{yy}$
- The bending stress at point A;
- The bending stress at point D;
- The orientation of the neutral axis on the cross-section. (illustrate with a sketch)

**Solution:**

a)

$$I_{zz} = \frac{10 \cdot 6^3}{12} - 2 \frac{\pi}{4} (1'')^4 = 178.4 \text{ in}^4$$

$$I_{yy} = \frac{6 \cdot 10^3}{12} - 2 \pi (2.5 \text{ in})^2 - 2 \frac{\pi}{4} (1'')^4 = 459.2 \text{ in}^4$$

b)

$$M_y = 60 \text{ k} \cdot \text{in} \cos 40^\circ = 45.96 \text{ k} \cdot \text{in}$$

$$M_z = -60 \text{ k} \cdot \text{in} \sin 40^\circ = -38.57 \text{ k} \cdot \text{in}$$

$$\sigma_A = \frac{M_y z_A}{I_{yy}} - \frac{M_z y_A}{I_{zz}} = \frac{(45.96 \text{ k} \cdot \text{in})(5'')}{459.17 \text{ in}^4} - \frac{(-38.57 \text{ k} \cdot \text{in})(3'')}{178.4 \text{ in}^4}$$

$$= 1.15 \text{ ksi}$$

c)

$$\sigma_D = \frac{M_y z_D}{I_{yy}} - \frac{M_z y_D}{I_{zz}} = \frac{(45.96 \text{ k} \cdot \text{in})(-5'')}{459.17 \text{ in}^4} - \frac{(-38.57 \text{ k} \cdot \text{in})(-3'')}{178.4 \text{ in}^4}$$

$$= -1.15 \text{ ksi}$$

d)

$$\sigma(y, z) = \frac{M_y}{I_{yy}} z - \frac{M_z}{I_{zz}} y = 0 \text{ (along the NA)}$$

$$z = -2.16y \text{ (equation for the NA)}$$

$$z = \tan(\alpha)y \text{ where } \alpha = \tan^{-1}(-2.16) = -65.16^\circ$$