Fluid Element Kinematics

Fluid element motion consists of translation, linear deformation, rotation, and angular deformation.

Linear Motion and Deformation:

Linear deformation of a fluid element

Change in $\delta \forall$:

$$
\delta \forall = \left(\frac{\partial u}{\partial x} \delta x\right) \left(\delta y \delta z\right) \delta t
$$

the rate at which the volume $\delta \forall$ is changing per unit volume due to the gradient ∂u/∂x is

$$
\frac{1}{\delta \forall} \frac{d(\delta \forall)}{dt} = \lim_{\delta t \to 0} \left[\frac{(\partial u/\partial x) \delta t}{\delta t} \right] = \frac{\partial u}{\partial x}
$$

If velocity gradients ∂v/∂y and ∂w/∂z are also present, then using a similar analysis it follows that, in the general case,

$$
\frac{1}{\delta \nabla} \frac{d(\delta \nabla)}{dt} = \frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} = \nabla \cdot \mathbf{V}
$$

This rate of change of the volume per unit volume is called the volumetric dilatation rate.

Angular Motion and Deformation

For simplicity we will consider motion in the x–y plane, but the results can be readily extended to the more general case.

Angular motion and deformation of a fluid element

The angular velocity of line *OA*, ω*OA*, is

$$
\omega_{OA} = \lim_{\delta t \to 0} \frac{\delta \alpha}{\delta t}
$$

For small angles

ngles
\n
$$
\tan \delta \alpha \approx \delta \alpha = \frac{(\partial v/\partial x) \delta x \delta t}{\delta x} = \frac{\partial v}{\partial x} \delta t
$$

so that

$$
\omega_{OA} = \lim_{\delta t \to 0} \left[\frac{(\partial v/\partial x) \delta t}{\delta t} \right] = \frac{\partial v}{\partial x}
$$

Note that if ∂ *v*/∂*x* is positive, ω_{*OA*} will be counterclockwise.

Similarly, the angular velocity of the line *OB* is

$$
\omega_{OB} = \lim_{\delta t \to 0} \frac{\delta \beta}{\delta t} = \frac{\partial u}{\partial y}
$$

In this instance if ∂*u*/∂*y* is positive, ω*OB* will be clockwise.

The rotation, ω*^z* , of the element about the *z* axis is defined as the average of the angular velocities ω_{OA} and ω_{OB} of the two mutually perpendicular lines *OA* and *OB*. Thus, if counterclockwise rotation is considered to be positive, it follows that

$$
\omega_z = \frac{1}{2} \left(\frac{\partial v}{\partial x} - \frac{\partial u}{\partial y} \right)
$$

Rotation of the field element about the other two coordinate axes can be obtained in a similar manner:

$$
\omega_x = \frac{1}{2} \left(\frac{\partial w}{\partial y} - \frac{\partial v}{\partial z} \right)
$$

$$
\omega_y = \frac{1}{2} \left(\frac{\partial u}{\partial z} - \frac{\partial w}{\partial x} \right)
$$

The three components, ω_{x}, ω_{y} , and ω_{z} can be combined to give the rotation vector, **ω**, in the form:
 $\omega = \omega \mathbf{i} + \omega \mathbf{j} + \omega \mathbf{k} = -\frac{1}{2} \omega r / \mathbf{V} = -\frac{1}{2} \omega r / \omega$

rotation vector,
$$
\omega
$$
, in the form:
\n
$$
\omega = \omega_x \mathbf{i} + \omega_y \mathbf{j} + \omega_z \mathbf{k} = \frac{1}{2} \text{curl} \mathbf{V} = \frac{1}{2} \nabla \times \mathbf{V}
$$

since

$$
\frac{1}{2}\nabla \times \mathbf{V} = \frac{1}{2} \begin{vmatrix} \mathbf{i} & \mathbf{j} & \mathbf{k} \\ \frac{\partial}{\partial x} & \frac{\partial}{\partial y} & \frac{\partial}{\partial z} \\ u & v & w \end{vmatrix}
$$

$$
= \frac{1}{2} \left(\frac{\partial w}{\partial y} - \frac{\partial v}{\partial z} \right) \mathbf{i} + \frac{1}{2} \left(\frac{\partial u}{\partial z} - \frac{\partial w}{\partial x} \right) \mathbf{j} + \frac{1}{2} \left(\frac{\partial v}{\partial x} - \frac{\partial u}{\partial y} \right) \mathbf{k}
$$

The vorticity, **ζ**, is defined as a vector that is twice the rotation vector; that is,

$$
\varsigma = 2\omega = \nabla \times \mathbf{V}
$$

The use of the vorticity to describe the rotational characteristics of the fluid simply eliminates the (1/2) factor associated with the rotation vector. If $\nabla \times \mathbf{V} = 0$, the flow is called irrotational.

In addition to the rotation associated with the derivatives ∂u/∂y and ∂v/∂x, these derivatives can cause the fluid element to undergo an angular deformation, which results in a change in shape of the element. The change in the original right angle formed by the lines OA and OB is termed the shearing strain, δγ,

$$
\delta \gamma = \delta \alpha + \delta \beta
$$

The rate of change of δy is called the rate of shearing strain or the rate of angular deformation:

$$
\dot{\gamma}_{xy} = \lim_{\delta t \to 0} \frac{\delta \gamma}{\delta t} = \lim_{\delta t \to 0} \frac{\delta \gamma}{\delta t} = \left[\frac{(\partial v / \partial x) \delta t + (\partial u / \partial y) \delta t}{\delta t} \right] = \frac{\partial v}{\partial x} + \frac{\partial u}{\partial y}
$$

Similarly,

$$
\dot{\gamma}_{xz} = \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z}
$$

$$
\dot{\gamma}_{yz} = \frac{\partial w}{\partial y} + \frac{\partial v}{\partial z}
$$

The rate of angular deformation is related to a corresponding shearing stress which causes the fluid element to change in shape.

The Continuity Equation in Differential Form

The governing equations can be expressed in both integral and differential form. Integral form is useful for large-scale control volume analysis, whereas the differential form is useful for relatively small-scale point analysis.

Application of RTT to a fixed elemental control volume yields the differential form of the governing equations. For example for conservation of mass

$$
\sum_{CS} \rho \underline{V} \cdot \underline{A} = -\int_{CV} \frac{\partial \rho}{\partial t} d\Psi
$$

net outflow of mass \equiv rate of decrease across CS of mass within CV

Consider a cubical element oriented so that its sides are || to
\nthe
$$
(x,y,z)
$$
 axes
\n
$$
\left[\rho u + \frac{\partial}{\partial x}(\rho u)dx\right]dydz
$$
\n
$$
\left[\rho u + \frac{\partial}{\partial x}(\rho u)dx\right]dydz
$$
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$$
\left[\rho u + \frac{\partial}{\partial x}(\rho u)dx\right]dydz
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\left[\rho u + \frac{\partial}{\partial x}(\rho u)dx\right]dydz
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\left[\rho u + \frac{\partial}{\partial x}(\rho u)dx\right]dydz
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$$
\left[\rho u + \frac{\partial}{\partial x}(\rho u)dx\right]dydz
$$
\n
$$
\left[\rho u + \frac{\partial}{\partial x}(\rho u)dx\right]dydz
$$

We assume that the element is infinitesimally small such that we can assume that the flow is approximately one dimensional through each face.

The mass flux terms occur on all six faces, three inlets, and three outlets. Consider the mass flux on the x faces

$$
x_{\text{flux}} = \left[\rho u + \frac{\partial}{\partial x}(\rho u) dx\right] dydz\Big|_{\text{outflux}} - \rho u dydz\Big|_{\text{influx}}
$$

$$
= \frac{\partial}{\partial x}(\rho u) dx dydz
$$

Similarly for the y and z faces

$$
y_{flux} = \frac{\partial}{\partial y} (\rho v) dxdydz
$$

$$
z_{flux} = \frac{\partial}{\partial z} (\rho w) dxdydz
$$

The total net mass outflux must balance the rate of decrease of mass within the CV which is

$$
-\frac{\partial \rho}{\partial t}dxdydz
$$

Combining the above expressions yields the desired result

$$
\left[\frac{\partial \rho}{\partial t} + \frac{\partial}{\partial x}(\rho u) + \frac{\partial}{\partial y}(\rho v) + \frac{\partial}{\partial z}(\rho w)\right] dxdydz = 0
$$

$$
\frac{\partial \rho}{\partial t} + \frac{\partial}{\partial x} (\rho u) + \frac{\partial}{\partial y} (\rho v) + \frac{\partial}{\partial z} (\rho w) = 0
$$

per unit V differential form of continuity equations

$$
\frac{\partial \rho}{\partial t} + \nabla \cdot (\rho \underline{V}) = 0
$$

$$
\rho \nabla \cdot \underline{V} + \underline{V} \cdot \nabla \rho
$$

 $+\rho \nabla \cdot \underline{V} =$

 $\underline{V} = 0$

$$
\frac{D}{Dt} = \frac{\partial}{\partial t} + \underline{V} \cdot \nabla
$$

Nonlinear 1st order PDE; (unless ρ = constant, then linear) Relates \underline{V} to satisfy kinematic condition of mass conservation

Simplifications:

Dt

D

 ρ

- 1. Steady flow: $\nabla \cdot (\rho \underline{V}) = 0$
- 2. $p = constant: \nabla \cdot \underline{V} = 0$

i.e.,
$$
\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} = 0
$$
 3D

$$
\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} = 0 \qquad 2D
$$

The continuity equation in Cylindrical Polar Coordinates

The velocity at some arbitrary point P can be expressed as $\mathbf{V} = v_r \mathbf{e}_r + v_\theta \mathbf{e}_\theta + v_z \mathbf{e}_z$

The continuity equation:
\n
$$
\frac{\partial \rho}{\partial t} + \frac{1}{r} \frac{\partial (r \rho v_r)}{\partial r} + \frac{1}{r} \frac{\partial (\rho v_\theta)}{\partial \theta} + \frac{\partial (\rho v_z)}{\partial z} = 0
$$

For steady, compressible flow
\n
$$
\frac{1}{r} \frac{\partial (r \rho v_r)}{\partial r} + \frac{1}{r} \frac{\partial (\rho v_\theta)}{\partial \theta} + \frac{\partial (\rho v_z)}{\partial z} = 0
$$

For incompressible fluids (for steady or unsteady flow)

$$
\frac{1}{r}\frac{\partial (r v_r)}{\partial r} + \frac{1}{r}\frac{\partial v_\theta}{\partial \theta} + \frac{\partial v_z}{\partial z} = 0
$$

The Stream Function

Steady, incompressible, plane, two-dimensional flow represents one of the simplest types of flow of practical importance. By plane, two-dimensional flow we mean that there are only two velocity components, such as u and v, when the flow is considered to be in the x-y plane. For this flow the continuity equation reduces to

We still have two variables, u and v, to deal with, but they must be related in a special way as indicated. This equation suggests that if we define a function $\psi(x, y)$, called the stream function, which relates the velocities as

$$
u = \frac{\partial \psi}{\partial y}, \quad v = -\frac{\partial \psi}{\partial x}
$$

then the continuity equation is identically satisfied:
 $\partial (\partial w)$ $\partial (\partial w)$ $\partial^2 w$ $\partial^2 w$

continuity equation is identically satisfied:
\n
$$
\frac{\partial}{\partial x} \left(\frac{\partial \psi}{\partial y} \right) + \frac{\partial}{\partial y} \left(-\frac{\partial \psi}{\partial x} \right) = \frac{\partial^2 \psi}{\partial x \partial y} - \frac{\partial^2 \psi}{\partial x \partial y} = 0
$$

Velocity and velocity components along a streamline

Another particular advantage of using the stream function is related to the fact that lines along which ψ is constant are streamlines. The change in the value of ψ as we move from one point (x, y) to a nearby point $(x + dx, y + dy)$ along a ψ is given by the
 $\frac{\partial \psi}{\partial x} dx + \frac{\partial \psi}{\partial y} dy =$

line of constant
$$
\psi
$$
 is given by the relationship:
\n
$$
d\psi = \frac{\partial \psi}{\partial x} dx + \frac{\partial \psi}{\partial y} dy = -v dx + u dy = 0
$$

and, therefore, along a line of constant ψ

The flow between two streamlines

The actual numerical value associated with a particular streamline is not of particular significance, but the change in the value of ψ is related to the volume rate of flow. Let dq represent the volume rate of flow (per unit width perpendicular to the x–y plane) passing between the two streamlines.

es.
\n
$$
dq = u dy - v dx = \frac{\partial \psi}{\partial x} dx + \frac{\partial \psi}{\partial y} dy = d\psi
$$

Thus, the volume rate of flow, q, between two streamlines such as ψ 1 and ψ 2, can be determined by integrating to yield:

$$
q=\int_{\psi_1}^{\psi_2}d\psi=\psi_2-\psi_1
$$

In cylindrical coordinates the continuity equation for incompressible, plane, two-dimensional flow reduces to

and the velocity components, v_r and v_θ , can be related to the stream function, $\psi(r, \theta)$, through the equations

$$
v_r = \frac{1}{r} \frac{\partial \psi}{\partial \theta}, \quad v_\theta = -\frac{\partial \psi}{\partial r}
$$

Navier-Stokes Equations

Differential form of momentum equation can be derived by applying control volume form to elemental control volume

The differential equation of linear momentum: elemental fluid volume approach

combining and making use of the continuity equation yields

$$
\Sigma \underline{F} = \left[\underline{V} \left\{ \frac{\partial \rho}{\partial t} + \nabla \cdot (\rho \underline{V}) \right\} + \rho \left(\frac{\partial \underline{V}}{\partial t} + \underline{V} \cdot \nabla \underline{V} \right) \right] dxdydz
$$

\n
$$
\therefore \sum \underline{F} = \rho \frac{D\underline{V}}{Dt} dxdydz \text{ or } \sum \underline{f} = \rho \frac{D\underline{V}}{Dt}
$$

\nwhere $\Sigma \underline{F} = \Sigma \underline{F}_{\text{body}} + \Sigma \underline{F}_{\text{surface}}$
\n
$$
\Sigma \underline{f} = \Sigma \underline{f}_{\text{body}} + \Sigma \underline{f}_{\text{surface}}
$$

\n
$$
\Sigma \underline{f} = \Sigma \underline{f}_{\text{body}} + \Sigma \underline{f}_{\text{surface}}
$$

Body forces are due to external fields such as gravity or magnetics. Here we only consider a gravitational field; that is,

$$
\sum E_{body} = dE_{grav} = \rho g dx dy dz
$$

and $g = -g\hat{k}$ for $g\hat{k}$ z[†]
i.e., $f_{body} = -\rho g\hat{k}$

Surface forces are due to the stresses that act on the sides of the control surfaces

As shown before for p alone it is not the stresses themselves that cause a net force but their gradients.

$$
dF_{x, surf} = \left[\frac{\partial}{\partial x} (\sigma_{xx}) + \frac{\partial}{\partial y} (\sigma_{xy}) + \frac{\partial}{\partial z} (\sigma_{xz}) \right] dxdydz
$$

$$
= \left[-\frac{\partial p}{\partial x} + \frac{\partial}{\partial x} (\tau_{xx}) + \frac{\partial}{\partial y} (\tau_{xy}) + \frac{\partial}{\partial z} (\tau_{xz}) \right] dxdydz
$$

$$
\underline{\tau}_x = \tau_{xx} \hat{i} + \tau_{xy} \hat{j} + \tau_{xz} \hat{k}
$$

and noting that

$$
dF_{x, surf} = \left[-\frac{\partial p}{\partial x} + \nabla \cdot \underline{\tau}_x \right] dxdydz
$$

$$
f_{x, surf} = -\frac{\partial p}{\partial x} + \nabla \cdot \underline{\tau}_x \quad \text{per unit volume}
$$

similarly for y and z

$$
f_{y,surf}\ =\ -\frac{\partial p}{\partial y} + \nabla\cdot \underline{\tau}_y \qquad \underline{\tau}_y = \tau_{yx}\hat{i} + \tau_{yy}\hat{j} + \tau_{yz}\hat{k}
$$

$$
f_{z,surf}\ =\ -\frac{\partial p}{\partial z}+\nabla\cdot \underline{\tau}_z \qquad \underline{\tau}_z=\tau_{zx}\hat{i}+\tau_{zy}\hat{j}+\tau_{zz}\hat{k}
$$

finally if we define

$$
\tau_{ij} = \underline{\tau}_x \hat{i} + \underline{\tau}_y \hat{j} + \underline{\tau}_z \hat{k} \qquad \text{then}
$$

$$
\underline{\mathbf{f}}_{\text{surf}} = -\nabla p + \nabla \cdot \tau_{ij} = \nabla \cdot \sigma_{ij} \qquad \qquad \sigma_{ij} = -p\delta_{ij} + \tau_{ij}
$$

Putting together the above results

$$
\sum \underline{f} = \underline{f}_{body} + \underline{f}_{surf} = \rho \frac{D\underline{V}}{Dt}
$$

$$
\underline{\mathbf{f}}_{body} = -\rho g \hat{\mathbf{k}}
$$
\n
$$
\underline{\mathbf{f}}_{surface} = -\nabla p + \nabla \cdot \tau_{ij}
$$
\n
$$
\underline{\mathbf{a}} = \frac{D \mathbf{V}}{Dt} = \frac{\partial \mathbf{V}}{\partial t} + \underline{\mathbf{V}} \cdot \nabla \mathbf{V}
$$

For Newtonian fluid the shear stress is proportional to the rate of strain, which for incompressible flow can be written

$$
\tau_{ij} = 2\mu\varepsilon_{ij} = \mu \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right)
$$

where,

$$
\mu = \text{coefficient of viscosity}
$$
\n
$$
\varepsilon_{ij} = \text{rate of strain tensor}
$$
\n
$$
= \begin{bmatrix}\n\frac{\partial u}{\partial x} & \frac{1}{2} \left(\frac{\partial v}{\partial x} + \frac{\partial u}{\partial y} \right) & \frac{1}{2} \left(\frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \right) \\
\frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) & \frac{\partial v}{\partial y} & \frac{1}{2} \left(\frac{\partial w}{\partial y} + \frac{\partial v}{\partial z} \right) \\
\frac{1}{2} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) & \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) & \frac{\partial w}{\partial z}\n\end{bmatrix}
$$
\n
$$
\rho \underline{a} = -\rho g \hat{k} - \nabla p + \nabla \cdot (\tau_{ij})
$$
\n
$$
\rho = -\rho g \hat{k} - \nabla p + \nabla \cdot (\tau_{ij})
$$

where,

$$
\nabla \cdot (\tau_{ij}) = \mu \frac{\partial}{\partial x_j} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) = \mu \left(\frac{\partial^2 u_i}{\partial x_j^2} + \frac{\partial}{\partial x_i} \frac{\partial u_j}{\partial x_j} \right)
$$

٦

$$
\rho \underline{a} = -\rho g \hat{k} - \nabla p + \mu \nabla^2 \underline{V}
$$

$$
\rho \underline{a} = -\nabla (p + \gamma z) + \mu \nabla^2 \underline{V}
$$

$$
\nabla \cdot \underline{V} = 0
$$

 Navier-Stokes Equation Continuity Equation

Four equations in four unknowns: V and p
\nDifficult to solve since
$$
2^{nd}
$$
 order nonlinear PDE
\nx: $\rho \left[\frac{\partial u}{\partial t} + u \frac{\partial u}{\partial x} + v \frac{\partial u}{\partial y} + w \frac{\partial u}{\partial z} \right] = -\frac{\partial p}{\partial x} + \mu \left[\frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 u}{\partial y^2} + \frac{\partial^2 u}{\partial z^2} \right]$
\ny: $\rho \left[\frac{\partial v}{\partial t} + u \frac{\partial v}{\partial x} + v \frac{\partial v}{\partial y} + w \frac{\partial v}{\partial z} \right] = -\frac{\partial p}{\partial y} + \mu \left[\frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 v}{\partial y^2} + \frac{\partial^2 v}{\partial z^2} \right]$
\nz: $\rho \left[\frac{\partial w}{\partial t} + u \frac{\partial w}{\partial x} + v \frac{\partial w}{\partial y} + w \frac{\partial w}{\partial z} \right] = -\frac{\partial p}{\partial z} - \rho g + \mu \left[\frac{\partial^2 w}{\partial x^2} + \frac{\partial^2 w}{\partial y^2} + \frac{\partial^2 w}{\partial z^2} \right]$
\n $\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} = 0$

Navier-Stokes equations can also be written in other coordinate systems such as cylindrical, spherical, etc.

There are about 80 exact solutions for simple geometries. For practical geometries, the equations are reduced to algebraic form using finite differences and solved using computers.

Ex) Exact solution for laminar incompressible steady flow in a circular pipe

Use cylindrical coordinates with assumptions

$$
\frac{\partial}{\partial z} = 0
$$
: Fully-developed flow

$$
v_{\theta} = 0
$$
: Flow is parallel to the wall

Continuity equation:

$$
\frac{1}{r}\frac{\partial (rv_r)}{\partial r} + \frac{1}{r}\frac{\partial v_{\theta}^{\pi}}{\partial \theta} + \frac{\partial v_{z}^{\pi}}{\partial z} = 0
$$

$$
v_r = \frac{c}{r}
$$

B.C. $v_r(r = 0) = 0 \implies C = 0$

i.e., $v_r = 0$

$$
\rho \left(\frac{\partial v_r}{\partial t} + v_r \frac{\partial v_r}{\partial r} + \frac{v_\theta}{r} \frac{\partial v_r}{\partial \theta} - \frac{v_\theta^2}{r} + v_z \frac{\partial v_r}{\partial z} \right) \n= -\frac{\partial p}{\partial r} + \rho g_r + \mu \left[\frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial v_r}{\partial r} \right) - \frac{v_r}{r^2} + \frac{1}{r^2} \frac{\partial^2 v_r}{\partial \theta^2} - \frac{2}{r^2} \frac{\partial v_\theta}{\partial \theta} + \frac{\partial^2 v_r}{\partial z^2} \right] \n\rho \left(\frac{\partial v_\theta}{\partial t} + v_r \frac{\partial v_\theta}{\partial r} + \frac{v_\theta}{r} \frac{\partial v_\theta}{\partial \theta} + \frac{v_r v_\theta}{r} + v_z \frac{\partial v_\theta}{\partial z} \right) \n= -\frac{\partial p}{\partial \theta} + \rho g_\theta + \mu \left[\frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial v_\theta}{\partial r} \right) - \frac{v_\theta}{r^2} + \frac{1}{r^2} \frac{\partial^2 v_\theta}{\partial \theta^2} + \frac{2}{r^2} \frac{\partial v_r}{\partial \theta} + \frac{\partial^2 v_\theta}{\partial z^2} \right] \n\rho \left(\frac{\partial v_z}{\partial t} + v_r \frac{\partial z}{\partial r} + \frac{v_\theta}{r} \frac{\partial v_z}{\partial \theta} + v_z \frac{\partial v_z}{\partial z} \right) \n= -\frac{\partial p}{\partial z} + \rho g_z + \mu \left[\frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial v_z}{\partial r} \right) + \frac{1}{r^2} \frac{\partial^2 v_z}{\partial \theta^2} + \frac{\partial^2 v_z}{\partial z^2} \right]
$$

or

$$
0 = -\rho g \sin \theta - \frac{\partial p}{\partial r}
$$
 (1)
\n
$$
0 = -\rho g \cos \theta - \frac{1}{r} \frac{\partial p}{\partial \theta}
$$
 (2)
\n
$$
0 = -\frac{\partial p}{\partial z} + \mu \left[\frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial v_z}{\partial r} \right) \right]
$$
 (3)

where,

$$
g_r = -g \sin \theta
$$

$$
g_\theta = -g \cos \theta
$$

Equations (1) and (2) can be integrated to give

$$
p = -\rho g(r \sin \theta) + f_1(z) = -\rho gy + f_1(z)
$$

 \Rightarrow pressure p is hydrostatic and $\partial p / \partial z$ is not a function of r or θ

Equation (3) can be written in the from

$$
\frac{1}{r}\frac{\partial}{\partial r}\left(r\frac{\partial v_z}{\partial r}\right) = \frac{1}{\mu}\frac{\partial p}{\partial z}
$$

and integrated (using the fact that $\partial p / \partial z =$ constant) to give

$$
r\frac{\partial v_z}{\partial r} = \frac{1}{2\mu} \left(\frac{\partial p}{\partial z}\right) r^2 + C_1
$$

Integrating again we obtain

$$
v_z = \frac{1}{4\mu} \left(\frac{\partial p}{\partial z}\right) r^2 + C_1 \ln r + C_2
$$

B.C.

$$
v_z(r = 0) \neq \infty \implies C_1 = 0
$$

$$
v_z(r = R) = 0 \implies C_2 = -\frac{1}{4\mu} \left(\frac{\partial p}{\partial z}\right) R^2
$$

$$
\therefore v_z = \frac{1}{4\mu} \left(\frac{\partial p}{\partial z}\right) (r^2 - R^2)
$$

$$
\therefore v_z = \frac{1}{4\mu} \left(\frac{\delta P}{\partial z}\right) (r^2 - R^2)
$$

 \Rightarrow at any cross section the velocity distribution is parabolic

1) Flow rate Q :

$$
Q = \int_0^R v_z dA = 2\pi \int_0^R v_z r dr = -\frac{\pi R^4}{8\mu} \left(\frac{\partial p}{\partial z}\right)
$$

where, $dA = (2\pi r)dr$

If the pressure drops Δp over a length ℓ : $\frac{\Delta}{\ell}$ ℓ $=-\frac{\partial}{\partial}$ ∂

$$
Q = \frac{\pi R^4 \Delta p}{8\mu\ell}
$$

2) Mean velocity V :

$$
V = \frac{Q}{A} = \left(\frac{1}{\pi R^2}\right) \left(\frac{\pi R^4 \Delta p}{8\mu \ell}\right) = \frac{R^2 \Delta p}{8\mu \ell}
$$

3) Maximum velocity v_{max} :

$$
v_{max} = v_z(r=0) = -\frac{R^2}{4\mu} \left(\frac{\partial p}{\partial z}\right) = \frac{R^2 \Delta p}{4\mu \ell} = 2V
$$

 \Rightarrow

$$
\frac{v_z}{v_{max}} = 1 - \left(\frac{r}{R}\right)^2
$$

4) Wall shear stress $(\tau_{rz})_{\text{wall}}$:

$$
\tau_{rz} = \mu \left(\frac{\partial v_r}{\partial z} + \frac{\partial v_z}{\partial r} \right) = \mu \frac{\partial v_z}{\partial r}
$$

where

$$
\frac{\partial v_z}{\partial r} = v_{\frac{max}{2V}} \left(-\frac{2r}{R^2} \right) = -\frac{4Vr}{R^2}
$$

Thus, at the wall (i.e., $r = R$),

$$
(\tau_{rz})_{wall} = -\frac{4\mu V}{R}
$$

and with $Q = \pi R^2 V$,

$$
|(\tau_{rz})_{wall}| = \frac{4\mu Q}{\pi R^3}
$$

Note: Only valid for laminar flows. In general, the flow remains laminar for Reynolds numbers, $Re = \rho V(2R)/\mu$, below 2100. Turbulent flow in tubes is considered in Chapter 8.

Differential Analysis of Fluid Flow

We now discuss a couple of exact solutions to the Navier-Stokes equations. Although all known exact solutions (about 80) are for highly simplified geometries and flow conditions, they are very valuable as an aid to our understanding of the character of the NS equations and their solutions. Actually the examples to be discussed are for internal flow (Chapter 8) and open channel flow (Chapter 10), but they serve to underscore and display viscous flow. Finally, the derivations to follow utilize differential analysis. See the text for derivations using CV analysis.

Couette Flow

First, consider flow due to the relative motion of two parallel plates

Continuity 0 x u $=$ ∂ ∂ Momentum 2 2 dy d^2u $0 = \mu$ $u = u(y)$ $v = 0$ 0 y p x p $=$ ∂ ∂ $=$ ∂ ∂

or by CV continuity and momentum equations:

$$
ρu1Δy = ρu2Δy
$$
\n
$$
u1 = u2
$$
\n
$$
\sum F_x = \sum uρ \sum \cdot dΔ = ρQ(u2 - u1) = 0
$$
\n
$$
= pΔy - (p + \frac{dp}{dx}Δx)Δy - τΔx + (τ + \frac{dT}{dy}dy)Δx = 0
$$
\n
$$
\frac{dτ}{dy} = 0
$$
\ni.e. $\frac{d}{dy} (μ \frac{du}{dy}) = 0$ \n
$$
μ \frac{d2u}{dy2} = 0
$$
\nfrom momentum equation\n
$$
μ \frac{du}{dy} = C
$$
\n
$$
u = \frac{C}{μ}y + D
$$
\n
$$
u(0) = 0 ⇒ D = 0
$$
\n
$$
u(t) = U ⇒ C = μ \frac{U}{t}
$$
\n
$$
u = \frac{U}{t}y
$$
\n
$$
τ = μ \frac{du}{dy} = \frac{μU}{t} = constant
$$
\n
$$
v = 0
$$
\

from momentum equation

dy

 $\mu \frac{u}{2} =$

2

0

$$
\mu \frac{du}{dy} = C
$$

\n
$$
u = \frac{C}{\mu} y + D
$$

\n
$$
u(0) = 0 \Rightarrow D = 0
$$

\n
$$
u(t) = U \Rightarrow C = \mu \frac{U}{t}
$$

\n
$$
u = \frac{U}{t} y
$$

\n
$$
\tau = \mu \frac{du}{dy} = \frac{\mu U}{t} = constant
$$

Generalization for inclined flow with a constant pressure gradient

Continuity
$$
\frac{\partial u}{\partial x} = 0
$$
 $u = u(y)$
\nMomentum $0 = -\frac{\partial}{\partial x} (p + \gamma z) + \mu \frac{d^2 u}{dy^2} \qquad \frac{\partial p}{\partial y} = 0$

i.e.,
$$
\mu \frac{d^2 u}{dy^2} = \gamma \frac{dh}{dx}
$$
 $h = p/\gamma + z = \text{constant}$
plates horizontal $\frac{dz}{dx} = 0$
plates vertical $\frac{dz}{dx} = -1$

which can be integrated twice to yield

$$
\mu \frac{du}{dy} = \gamma \frac{dh}{dx} y + A
$$

$$
\mu u = \gamma \frac{dh}{dx} \frac{y^2}{2} + Ay + B
$$

now apply boundary conditions to determine A and B $u(y = 0) = 0 \Rightarrow B = 0$ $u(y = t) = U$ 2 t dx dh t U $At \implies A$ 2 t dx $U = \gamma \frac{dh}{dt} \frac{t^2}{2}$ $-\gamma$ μ $\mu U = \gamma \frac{du}{dt} + At \implies A =$ Ω

$$
u(y) = \frac{\gamma}{\mu} \frac{dh}{dx} \frac{y^2}{2} + \frac{1}{\mu} \left[\frac{\mu U}{t} - \gamma \frac{dh}{dx} \frac{t}{2} \right]
$$

$$
= -\frac{\gamma}{2\mu} \frac{dh}{dx} (ty - y^2) + \frac{U}{t} y
$$

This equation can be put in non-dimensional form:

$$
\frac{u}{U} = -\frac{\gamma t^2}{2\mu U} \frac{dh}{dx} \left(1 - \frac{y}{t} \right) \frac{y}{t} + \frac{y}{t}
$$

define: $P = non-dimensional pressure gradient$

$$
= -\frac{\gamma t^2}{2\mu U} \frac{dh}{dx}
$$
\n
$$
Y = y/t
$$
\n
$$
\Rightarrow \frac{u}{U} = P \cdot Y(1 - Y) + Y
$$
\n
$$
= -\frac{\gamma z^2}{2\mu U} \left[\frac{1}{\gamma} \frac{dp}{dx} + \frac{dz}{dx} \right]
$$
\n
$$
\Rightarrow \frac{u}{U} = P \cdot Y(1 - Y) + Y
$$
\n
$$
\text{parabolic velocity profile}
$$

Fig. 5.2. Couette flow between two parallel flat walls $P > 0$, pressure decrease in direction of wall motion; $P < 0$, pressure increase; $P = 0$, zero pressure gradient

The maximum velocity occurs at the value of y for which:

$$
\frac{du}{dy} = 0 \qquad \frac{d}{dy} \left(\frac{u}{U} \right) = 0 = \frac{P}{t} - \frac{2P}{t^2}y + \frac{1}{t}
$$

$$
\Rightarrow y = \frac{t}{2P}(P+1) = \frac{t}{2} + \frac{t}{2P} \omega_{max}
$$

for
$$
U = 0
$$
, $y = t/2$

$$
\therefore u_{\text{max}} = u(y_{\text{max}}) = \frac{UP}{4} + \frac{U}{2} + \frac{U}{4P}
$$

note: if $U = 0$: 3 2 4 P 6 P u u max $=\frac{1}{2}$ $\frac{1}{4}$ =

The shape of the velocity profile $u(y)$ depends on P:

1. If P > 0, i.e., $\frac{du}{1}$ < 0 dx dh < 0 the pressure decreases in the direction of flow (favorable pressure gradient) and the

velocity is positive over the entire width

$$
\gamma \frac{dh}{dx} = \gamma \frac{d}{dx} \left(\frac{p}{\gamma} + z \right) = \frac{dp}{dx} - \gamma \sin \theta
$$

a)
$$
\frac{dp}{dx} < 0
$$

b)
$$
\frac{dp}{dx} < \gamma \sin \theta
$$

1. If $P < 0$, i.e., $dh/dx > 0$ the pressure increases in the direction of flow (adverse pressure gradient) and the velocity over a portion of the width can become negative (backflow) near the stationary wall. In this case the dragging action of the faster layers exerted on the fluid particles near the stationary wall is insufficient to overcome the influence of the adverse pressure gradient.

$$
\frac{dp}{dx} - \gamma \sin \theta > 0
$$
\n
$$
\frac{dp}{dx} > \gamma \sin \theta \qquad \text{or} \qquad \gamma \sin \theta < \frac{dp}{dx}
$$

2. If P = 0, i.e., $\frac{du}{1} = 0$ dx dh $= 0$ the velocity profile is linear y t U $u =$ a) $\frac{dp}{1} = 0$ dx dp $= 0$ and $\theta = 0$ Note: we derived b) $\frac{dp}{dp} = \gamma \sin \theta$ dx dp this special case

For U = 0 the form $\frac{u}{x} = PY(1 - Y) + Y$ U u $=$ PY(1 – Y) + Y is not appropriate $u = UPY(1-Y)+UY$ $=-\frac{r}{\epsilon} \frac{d\pi}{dt} Y(1-Y) + UY$ dx dh 2 t^2 $-{\rm Y}) +$ μ γ \overline{a} Now let $U = 0$: $u = -\frac{v}{\lambda} \frac{du}{dt} Y(1 - Y)$ dx dh 2 t u 2 \overline{a} μ γ $=$ $-$

3. Shear stress distribution

Non-dimensional velocity distribution

$$
u^* = \frac{u}{U} = P \cdot Y(1 - Y) + Y
$$

where $u^* \equiv \frac{u}{u}$ *U* $\equiv \frac{u}{U}$ is the non-dimensional velocity,

> 2 2 $P = -\frac{\gamma t^2}{2\pi r^2} \frac{dh}{dt}$ *U dx* γ μ $=-\frac{\gamma l}{2\mu I}\frac{dn}{dt}$ is the non-dimensional pressure gradient

y Y t $\equiv \frac{y}{t}$ is the non-dimensional coordinate.

Shear stress

$$
\tau = \mu \frac{du}{dy}
$$

In order to see the effect of pressure gradient on shear stress using the non-dimensional velocity distribution, we define the non-dimensional shear stress:

$$
\tau^* = \frac{\tau}{\frac{1}{2}\rho U^2}
$$

Then

$$
\tau^* = \frac{1}{\frac{1}{2}\rho U^2} \mu \frac{Ud(u/U)}{td(y/t)} = \frac{2\mu}{\rho Ut} \frac{du^*}{dY}
$$

$$
= \frac{2\mu}{\rho Ut} (-2PY + P + 1)
$$

$$
= \frac{2\mu}{\rho Ut} (-2PY + P + 1)
$$

$$
= A(-2PY + P + 1)
$$

where 2 $A = \frac{2\mu}{\sigma} > 0$ *Ut* μ ρ $\equiv \frac{2\mu}{\gamma L} > 0$ is a positive constant.

So the shear stress always varies linearly with *Y* across any section.

At the lower wall $(Y = 0)$. $\tau_{lw}^* = A(1+P)$ At the upper wall $(Y=1)$: $\tau_{uw}^* = A(1-P)$

For favorable pressure gradient, the lower wall shear stress is always positive:

1. For small favorable pressure gradient $(0 < P < 1)$:

$$
\tau_{lw}^* > 0 \text{ and } \tau_{uw}^* > 0
$$

2. For large favorable pressure gradient $(P > 1)$:

For adverse pressure gradient, the upper wall shear stress is always positive:

1. For small adverse pressure gradient $(-1 < P < 0)$:

$$
\tau_{lw}^* > 0 \text{ and } \tau_{uw}^* > 0
$$

2. For large adverse pressure gradient $(P < -1)$:

 τ_{lw}^* < 0 and τ_{uw}^* > 0

For $U = 0$, i.e., channel flow, the above non-dimensional form of velocity profile is not appropriate. Let's use dimensional form:

$$
u = -\frac{\gamma t^2}{2\mu} \frac{dh}{dx} Y(1-Y) = -\frac{\gamma}{2\mu} \frac{dh}{dx} y(t-y)
$$

Thus the fluid always flows in the direction of decreasing piezometric pressure or piezometric head because

0, $y > 0$ 2 $\frac{\gamma}{\gamma} > 0$, y μ > 0 , $y > 0$ and $t - y > 0$. So if *dh dx* is negative, *u* is positive; if *dh dx* is positive, *is negative.*

Shear stress:

$$
\tau = \mu \frac{du}{dy} = -\frac{\gamma}{2} \frac{dh}{dx} \left(t - \frac{1}{2} y \right)
$$

Since 1 0 2 $t-\frac{1}{2}y$ $\left(t-\frac{1}{2}y\right) > 0$, the sign of shear stress τ is always opposite to the sign of piezometric pressure gradient *dh* $\frac{dn}{dx}$, and the magnitude of τ is always maximum at both walls and zero at centerline of the channel.

Flow down an inclined plane

uniform flow \Rightarrow velocity and depth do not change in x-direction

Continuity
$$
\frac{du}{dx} = 0
$$

x-momentum
$$
0 = -\frac{\partial}{\partial x}(p + \gamma z) + \mu \frac{d^2 u}{dy^2}
$$

\ny-momentum $0 = -\frac{\partial}{\partial y}(p + \gamma z) \Rightarrow$ hydrostatic pressure variation
\n $\Rightarrow \frac{dp}{dx} = 0$
\n $\mu \frac{d^2 u}{dy^2} = -\gamma \sin \theta$
\n $\frac{du}{dy} = -\frac{\gamma}{\mu} \sin \theta y + c$
\n $u = -\frac{\gamma}{\mu} \sin \theta \frac{y^2}{2} + Cy + D$
\n $\frac{du}{dy}\Big|_{y=d} = 0 = -\frac{\gamma}{\mu} \sin \theta d + c \Rightarrow c = +\frac{\gamma}{\mu} \sin \theta d$
\n $u(0) = 0 \Rightarrow D = 0$
\n $u = -\frac{\gamma}{\mu} \sin \theta \frac{y^2}{2} + \frac{\gamma}{\mu} \sin \theta dy$
\n $= \frac{\gamma}{2\mu} \sin \theta y (2d - y)$

$$
u(y) = \frac{g \sin \theta}{2v} y(2d - y)
$$

\n
$$
q = \int_0^d u dy = \frac{\gamma}{2\mu} \sin \theta \left[dy^2 - \frac{y^3}{3} \right]_0^d
$$
discharge per unit width
\n
$$
= \frac{1}{3} \frac{\gamma}{\mu} d^3 \sin \theta
$$

\n
$$
\overline{V}_{avg} = \frac{q}{u} = \frac{1}{2} \frac{\gamma}{\mu} d^2 \sin \theta = \frac{gd^2}{2} \sin \theta
$$

 \mathbf{v}

3

in terms of the slope $S_0 = \tan \theta \sim \sin \theta$

$$
\overline{V} = \frac{gd^2S_o}{3v}
$$

 μ

3

d

Exp. show $\text{Re}_{\text{crit}} \sim 500$, i.e., for $\text{Re} \ge 500$ the flow will become turbulent

$$
\frac{\partial p}{\partial y} = -\gamma \cos \theta \qquad \qquad \text{Re}_{\text{crit}} = \frac{Vd}{v} \sim 500
$$

$$
p = -\gamma \cos \theta y + C
$$

$$
p(d) = p_o = -\gamma \cos \theta d + C
$$

i.e.,
$$
p = \gamma \cos \theta (d - y) + p_o
$$

$$
^*\;\; p(d)>p_o
$$

* if
$$
\theta = 0
$$
 $p = \gamma(d - y) + p_0$
entire weight of fluid imposed

if
$$
\theta = \pi/2
$$
 $p = p_0$
no pressure change through the fluid