

**53:134 Structural Design II (Steel Structures)**  
**Spring 2006 (Lecture Summary)**  
**Week 11 (3/27 - 3/31/06)**

**3/27/06**

- ◆ Design of tension members: Review of specifications.
  - Tension limit states - yielding of the section.
  - Fracture of the net section.
  - Slenderness ratio.
  - Effective area for fracture limit state - shear lag factor, reduction factor.
  - Block Shear, Section J4 of the Specs.
- ◆ AISC web-enhanced teaching animations; basics, tributary area, block shear, lateral loads.
- ◆ Design for tension - Examples. Calculations for U. Use of Tables: Part 1 - Section properties, Part 3 - design strength for yielding limit state and fracture limit state.
- ◆ **Read:** Chapters B and D of the AISC LRFD Specifications and Part 3 of the Manual, Design of Tension Members.

**3/29/06**

- ◆ Design of tension members - material from the textbook, Chapter 9, Section 9.1.1. Member failure modes:
  - Yielding failure of the gross cross-section.

- Fracture rupture of the net section.
- Shear rupture.
- Tension rupture.
- Shear-tension rupture.
- ◆ AISC animations:
  - Block shear rupture failure modes. Block shear capacity is the combined shear and tensile capacity of the failure mode (smallest capacity governs).
  - Block shear areas: Failure can happen in either yielding or fracture in all the modes. We calculate the yield capacity on the gross area and fracture capacity on the net area.
- ◆ Design of compression members - buckling failure is predominant and must be considered.
- ◆ AISC animations:
  - Slenderness ratio:  $KL/r$ ; effective length (braced, unbraced length).
  - Global buckling of members; Buckling happens when a member in compression becomes unstable due to its slenderness and load; elastic buckling (larger slenderness ratio, inelastic buckling smaller slenderness ratio).
  - Local buckling: instability is due to the plates of a member becoming unstable.
- ◆ **Midterm Exam 2 - Design of an Indeterminate Truss;**  
Due April 5, 2006.

**3/31/06**

- ◆ Design of compression members - buckling failure is predominant and must be considered.
- ◆ Design for compression: Global buckling of members; Euler buckling theory, buckling critical stress, slenderness ratio.
- ◆ Slenderness ratio:  $KL/r$ ; effective length (braced, unbraced length). Buckling happens when a member in compression becomes unstable due to its slenderness and load; elastic buckling (larger slenderness ratio, inelastic buckling smaller slenderness ratio).
- ◆ Local buckling: instability is due to the plates of a member becoming unstable; cross-sectional element slenderness is defined as  $b/t$  ratio; compact sections have small  $b/t$  and do not buckle locally; noncompact section can buckle locally; slender sections have a large  $b/t$  that require use LRFD Specifications Appendix E3. Limits for compact and noncompact sections, refer to Table B5.1.
- ◆ **Read:**
  - LRFD Specifications: Chapters B, E, Appendix B.
  - LRFD Manual: Part 4 Design of Compression Members.
- ◆ **Midterm Exam 2** - Due date extended to April 7, 2006.